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FLOOD INSURANCE STUDY



**CITY OF CEDARTOWN,
GEORGIA**

POLK COUNTY



COMPLETED

ORIGINAL

DECEMBER 16, 1988



Federal Emergency Management Agency

COMMUNITY NUMBER - 130153

NOTICE TO
FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

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FLOOD INSURANCE STUDY

CITY OF CEDARTOWN, POLK COUNTY, GEORGIA

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study revises and updates a previous Flood Insurance Study for the City of Cedartown, Polk County, Georgia (Reference 1). This information will be used by the community to update existing floodplain regulations as part of the regular phase of the National Flood Insurance Program. The information will also be used by local and regional planners to further promote sound land use and floodplain development.

In some states or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence and the State (or other jurisdictional agency) will be able to explain them.

This Flood Insurance Study was prepared by compiling existing hydrologic and hydraulic technical and scientific data prepared by the Federal Emergency Management Agency (FEMA) for purposes of the National Flood Insurance Program. The data were identified as the best available at the time of compilation of this Flood Insurance Study and should depict the general conditions of the flooding sources with relative accuracy. FEMA performed a cursory review and accepted the data as valid for purposes of this Flood Insurance Study and the National Flood Insurance Program. However, if better information is known to exist or has been developed since the date of this report, the information should be immediately forwarded to the Natural and Technological Hazards Division, FEMA, Atlanta, Georgia, for consideration for revision of this study.

1.2 Authority and Acknowledgments

The sources of authority for this Flood Insurance Study are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

The hydrologic and hydraulic analyses for this study were obtained from the Flood Insurance Study for the Unincorporated Areas of Polk County, Georgia (Reference 2).

1.3 Coordination

On April 10, 1984, a meeting attended by representatives of FEMA, Polk County, and Post, Buckley, Schuh & Jernigan Inc., was held. The primary purpose of this meeting was to identify streams requiring detailed study.

A second meeting was held on June 12, 1985, and was attended by representatives of Polk County and Post, Buckley, Schuh & Jernigan Inc. Information concerning historical flooding, local flow patterns, and community demographic data were exchanged.

On January 19, 1988, the results of this Flood Insurance Study were reviewed and accepted at a final coordination meeting attended by representatives of the community and FEMA.

2.0 AREA STUDIED

2.1 Scope of Study

This Flood Insurance Study covers the incorporated area of the City of Cedartown, Polk County, Georgia. The area of study is shown on the Vicinity Map (Figure 1).

The areas studied by detailed methods were selected based on the extent and validity of available existing hydrologic and hydraulic data.

Flooding caused by overflow of Cedar Creek, Tanyard Branch, Skeeter Branch, and South Prong Skeeter Branch was studied in detail within the community.

2.2 Community Description

Cedartown, the county seat of Polk County, is in northwestern Georgia, 61 miles northwest of Atlanta, Georgia, on U.S. Route 278 and is surrounded by the unincorporated areas of the county. The 1980 population of Cedartown was reported to be 8,619, a slight decrease from the 1970 population of 9,253 (Reference 3). Like many Georgia cities, new residential development in Cedartown is occurring adjacent to the city limits; therefore, future population growth will likely be a result of annexation rather than new development within the city.

Polk County was created by the State Legislature in 1851 from portions of Paulding and Floyd Counties that were formerly held by the Creek and Cherokee Indians. Cedartown, which is named for the sacred tree of the Cherokees, began as a trading post in 1826 and was incorporated in 1853.

The topography of Cedartown consists of low, rounded hills and wide valleys. The bottomland soils in the study area consist mainly of the Huntington, Pinson, Linside, and Melvin series. These soils are slightly to moderately erodible, with moderate permeability and low shrink-swell potential. The remainder of the soils are of the Minvale, Fullerton, Decatur, and Etowah series, which are moderately to highly erodible, with moderate permeability and a low to moderate shrink-swell potential.

Ground cover is primarily clover, bluegrass, fescue, ryegrass, blackberry, and honeysuckle. The trees are primarily shortleaf, loblolly, Virginia, and longleaf pines; southern red, white, blackjack, and post oaks; hickory; sweetgum; and eastern red cedar.

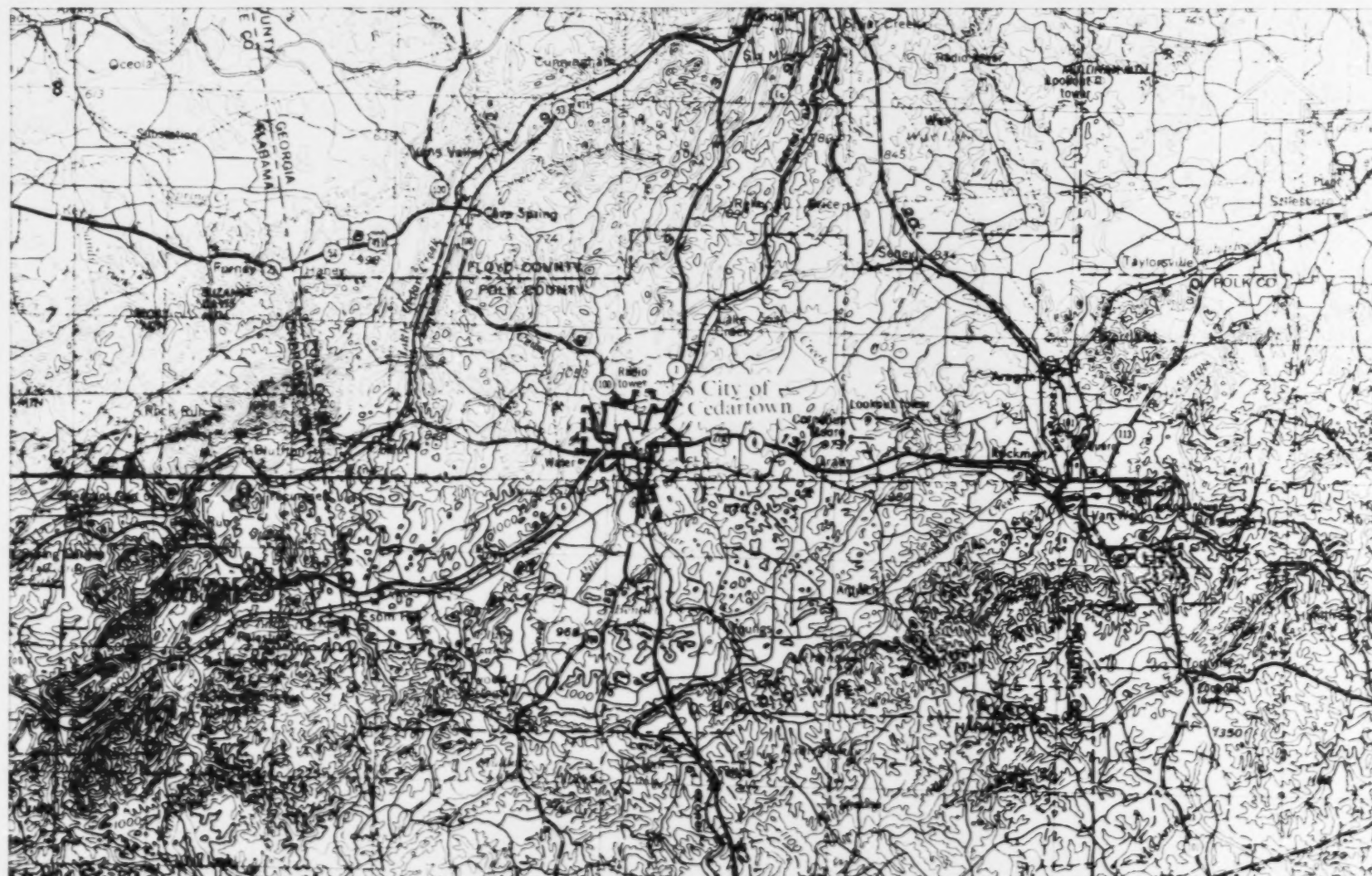


FIGURE 1

FEDERAL EMERGENCY MANAGEMENT AGENCY

CITY OF CEDARTOWN, GA
(POLK CO.)

APPROXIMATE SCALE

4 0 4 8 12 MILES

VICINITY MAP

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Mean monthly temperatures range from 42 degrees Fahrenheit (°F) during the winter to 76 °F during the summer. The mean annual rainfall is approximately 52 inches.

Cedartown is in the south-central portion of the Cedar Creek Watershed, which consists of a total of 208 square miles in Polk and Floyd Counties. Cedar Creek is a tributary to the Coosa River, with headwaters rising on Dug Down Mountain in Haralson County, Georgia, and Indian Mountain in Cherokee and Cleburne Counties, Alabama. The drainage area of the Cedar Creek watershed at Cedartown is approximately 109 square miles.

From its headwater, Cedar Creek flows northerly through Cedartown into Lake Weiss on the Coosa River. In addition to the portions of Cedar Creek in the city, the study includes Skeeter Branch, South Prong Skeeter Branch, Tanyard Branch, and all tributaries to Cedar Creek.

Many floodplains in the city are used for residential, commercial, industrial, and public purposes. Economic development in the area is expected to continue, resulting in pressure to develop land within the floodplain, particularly along Cedar Creek in the northern portion of the city.

2.3 Principal Flood Problems

Flooding of land adjacent to Cedar Creek and its main tributaries is frequent, due to the relatively high runoff, flat floodplain, and meandering stream channels.

Minor flooding, causing little or no property damage, occurs an average of two to three times per year, usually in late spring or summer as a result of thunderstorms.

Large-magnitude floods have occurred, causing damage to buildings, streets, and utilities. They are most likely to occur in the winter and early spring as a result of frontal systems. The highest recorded flood, which occurred in 1948, had a recurrence interval of 60 years. A flood in 1974 had a recurrence interval of 30 years. A 1979 flood caused extensive damage to residential, commercial, and municipal property (Reference 4).

2.4 Flood Protection Measures

The Cedar Creek Watershed Plan (Reference 4), which was inactive for many years, is now in phased construction. Two structures have been completed and a third is expected to be completed in 1989. Several other flood-retarding structures are proposed to reduce flood damage for the near future. A levee system is also proposed within the City of Cedartown.

Levees exist in the study area that provide the community with some degree of protection against flooding. However, it has been ascertained that these levees may not protect the community from rare events such as the 100-year flood. The criteria used to evaluate protection against the 100-year flood are 1) adequate design, including freeboard, 2) structural stability,

and 3) proper operation and maintenance. Levees that do not protect against the 100-year flood are not considered in the hydraulic analysis of the 100-year floodplain.

3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the community, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude that is expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood that equals or exceeds the 100-year flood (1-percent chance of annual exceedence) in any 50-year period is approximately 40 percent (4 in 10), and, for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for each riverine flooding source studied in detail affecting the community.

Cedar Creek has undergone recent hydrologic analyses by the U.S. Soil Conservation Service (SCS) as part of the development of the Cedar Creek Watershed Plan. The routed discharges developed by the SCS to account for the effects of flood-retarding structures on Cedar Creek were used. For the remaining streams studied by detailed methods, regional flood flow-frequency data were used. The regional regression equations adopted are those developed by the U.S. Geological Survey (USGS) (Reference 5). These equations are for rural undeveloped areas. Any adjustment required for urbanization was made using the techniques outlined by the USGS (Reference 6). A detailed discussion of the methodology used in the hydrologic analyses is presented in "Hydrology Study for Aragon, Rockmart and Unincorporated Polk County" (Reference 7).

Peak discharge-drainage area relationships for the 10-, 50-, 100-, and 500-year floods of each flooding source studied in detail in the community are shown in Table 1.

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals.

TABLE 1 - SUMMARY OF DISCHARGES

<u>FLOODING SOURCE AND LOCATION</u>	<u>DRAINAGE AREA</u>	<u>PEAK DISCHARGES (CFS)</u>			
	<u>SQ MILES</u>	<u>10-YEAR</u>	<u>50-YEAR</u>	<u>100-YEAR</u>	<u>500-YEAR</u>
CEDAR CREEK					
just upstream of confluence of Skeeter Branch	77.78	8,845	12,988	15,080	19,135
just upstream of confluence of Tanyard Branch	69.83	8,096	11,986	13,962	17,788
TANYARD BRANCH					
at Canal Street	6.18	1,500	2,060	2,300	2,800
at South Main Street	5.74	1,430	1,950	2,180	2,700
just upstream of Seaboard Coast Line Railroad	3.01	830	1,160	1,330	1,630
SKEETER BRANCH					
at College Street	6.04	1,438	2,130	2,454	3,241
just upstream of confluence of South Prong Skeeter Branch	4.00	895	1,364	1,566	2,105
SOUTH PRONG SKEETER BRANCH					
at mouth	2.16	781	1,138	1,313	1,705

Overbank cross sections for the backwater analyses of the streams studied in detail were obtained from aerial photographs (Reference 8). The below-water sections were obtained by field measurement. All bridges, dams, and culverts were field surveyed to obtain elevation data and structural geometry.

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles and on the Flood Insurance Rate Map.

Roughness factors (Manning's "n") used in the hydraulic computations were chosen by engineering judgment and based on field observations of the streams and floodplain areas. The roughness value for the main channel of Cedar Creek ranges from 0.03 to 0.05, and averages 0.045 for the majority of reaches. Channel roughness factors for Skeeter Branch and South Prong Skeeter Branch are slightly higher (0.03-0.06), primarily because of increased stream debris and rock observed during field reconnaissance. Roughness values for the overbank areas on those streams range from 0.035 where the floodplain is grassy pastureland and relatively free of obstructions, to 0.15 where there is heavy forest and underbrush. The roughness value for Tanyard Branch averages 0.05 for the channel and ranges from 0.065 to 0.200 for the overbank areas.

Water-surface elevations of floods of the selected recurrence intervals were computed using the HEC-2 step-backwater computer program (Reference 9). Starting water-surface elevations for the streams studied in detail were calculated using the slope-area method.

Flood profiles were drawn showing the computed water-surface elevations for floods of the selected recurrence intervals. In cases where the 50- and 100-year flood elevations are close together, due to limitations of the profile scale, only the 100-year profile has been shown.

The hydraulic analyses for this study are based on the effects of unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

All elevations are referenced to the National Geodetic Vertical Datum of 1929 (NGVD). Elevation reference marks used in this study are shown on the map and described in the exhibit labeled Elevation Reference Marks.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The National Flood Insurance Program encourages State and local governments to adopt sound floodplain management programs. Therefore, each Flood Insurance Study provides 100-year flood elevations and delineations of the 100- and 500-year floodplain boundaries and 100-year floodway to assist communities in developing floodplain management measures.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent annual chance (100-year) flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent annual chance (500-year) flood is employed to indicate additional areas of flood risk in the community. For each stream studied in detail, the 100- and 500-year floodplain boundaries have been delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated using topographic maps at a scale of 1:4800 with a contour interval of 4 feet (Reference 10).

The 100- and 500-year floodplain boundaries are shown on the Flood Insurance Rate Map. On this map, the 100-year floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A, AE, AH, AO, A99, V, and VE) and the 500-year floodplain boundary corresponds to the boundary of areas of moderate flood hazards. In cases where the 100- and 500-year floodplain boundaries are close together, only the 100-year floodplain boundary has been shown. Small areas within the floodplain boundaries may lie above the flood elevations but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces the flood-carrying capacity, increases the flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the National Flood Insurance Program, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 100-year floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 100-year flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this study were computed for certain stream segments on the basis of equal conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations are tabulated for selected cross sections and are shown in Table 2, Floodway Data. The computed floodways are shown on the Flood Insurance Rate Map. In cases where the floodway and the 100-year floodplain boundaries are either close together or collinear, only the floodway boundary is shown. Portions of the floodway for Cedar Creek lie outside the corporate limits.

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC.)	REGULATORY (FEET NGVD)	WITHOUT FLOODWAY (FEET NGVD)	WITH FLOODWAY (FEET NGVD)	INCREASE (FEET)
CEDAR CREEK								
A	37,900	1363 ⁴	10,148	1.5	768.7	768.7	769.6	0.9
B	43,550	911	8576	1.8	774.5	774.5	775.3	0.8
C	48,200	805	6761	2.1	778.2	778.2	779.0	0.8
TANYARD BRANCH								
A	0.62 ²	512	1463	1.6	777.4	777.4	777.4	0.0
B	0.71 ²	66	415	5.4	778.0	778.0	778.0	0.0
C	0.90 ²	326	1328	1.6	781.3	781.3	781.6	0.3
D	1.05 ²	130	801	2.7	781.8	781.8	782.8	1.0
E	1.23 ²	150	379	5.4	784.3	784.3	784.4	0.1
F	1.51 ²	313	1548	0.9	791.5	791.5	792.1	0.6
SKEETER BRANCH								
A	6,940 ³	494	1809	1.5	771.2	771.2	771.3	0.1
B	11,500 ³	180	874	2.8	782.8	782.8	783.3	0.5
C	14,910 ³	100	377	4.2	787.3	787.3	788.3	1.0
D	16,000 ³	266	690	2.0	791.3	791.3	792.3	1.0

¹FEET ABOVE COUNTY BOUNDARY

²MILES ABOVE CONFLUENCE WITH CEDAR CREEK

³FEET ABOVE MOUTH

⁴THIS WIDTH EXTENDS BEYOND CORPORATE LIMITS

TABLE 2

FEDERAL EMERGENCY MANAGEMENT AGENCY

CITY OF CEDARTOWN, GA
(POLK CO.)

FLOODWAY DATA

CEDAR CREEK—TANYARD BRANCH—SKEETER BRANCH

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC.)	REGULATORY (FEET NGVD)	WITHOUT FLOODWAY (FEET NGVD)	WITH FLOODWAY (FEET NGVD)	INCREASE (FEET)
SOUTH PRONG SKEETER BRANCH								
A	860	56	197	6.7	783.7	783.2 ²	783.9	0.7
B	1820	29	176	7.5	787.1	787.1	787.4	0.3

¹FEET ABOVE MOUTH

²ELEVATIONS WITHOUT CONSIDERING BACKWATER EFFECT FROM SKEETER BRANCH

TABLE 2

FEDERAL EMERGENCY MANAGEMENT AGENCY

CITY OF CEDARTOWN, GA

(POLK CO.)

FLOODWAY DATA

SOUTH PRONG SKEETER BRANCH

Along streams where floodways have not been computed, the community must ensure that the cumulative effect of development in the floodplain will not cause more than a 1.0-foot increase in the base flood elevations at any point within the community.

The area between the floodway and the 100-year floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 100-year flood more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 2.

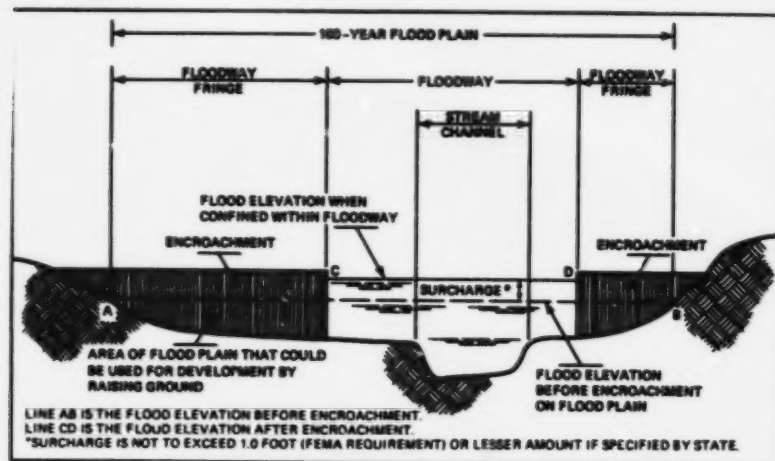


FIGURE 2 - Floodway Schematic

5.0 INSURANCE APPLICATION

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 100-year floodplains that are determined in the Flood Insurance Study by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no base flood elevations or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 100-year floodplains that are determined in the Flood Insurance Study by detailed methods. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

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Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 100-year floodplain, areas of 100-year flooding where average depths are less than 1 foot, areas of 100-year flooding where the contributing drainage area is less than 1 square mile, and areas protected from the 100-year flood by levees. No base flood elevations or depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The Flood Insurance Rate Map is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 100-year floodplains that were studied by detailed methods, shows selected whole-foot base flood elevations or average depths. Insurance agents use the zones and base flood elevations in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols the 100- and 500-year floodplains, the floodways, and the locations of selected cross sections used in the hydraulic analyses and floodway computations.

7.0 OTHER STUDIES

A Flood Insurance Study in progress for Polk County (Reference 2) is in agreement with this study.

This Flood Insurance Study supersedes the previously published Flood Insurance Study for the City of Cedartown (Reference 1).

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting the Natural and Technological Hazards Division, FEMA, 1371 Peachtree Street, NE., Suite 736, Atlanta, Georgia 30309.

9.0 REFERENCES AND BIBLIOGRAPHY

1. U.S. Department of Housing and Urban Development, Federal Insurance Administration, Flood Insurance Study, City of Cedartown, Polk County, Georgia, November 1977.
2. Federal Emergency Management Agency, Flood Insurance Study, Polk County, Unincorporated Areas, Georgia, in progress.
3. U.S. Department of Commerce, Bureau of the Census, 1980 Census of Population, Number of Inhabitants, Georgia, Washington, D.C., January 1982.
4. U.S. Department of Agriculture, Soil Conservation Service, Big Cedar Creek Watershed, Supplemental Watershed Plan, January 1984.

5. U.S. Department of the Interior, U.S. Geological Survey, Water Resources Investigations Report 78-137, Floods in Georgia, Magnitude and Frequency; Techniques for Estimating the Magnitude and Frequency of Floods in Georgia with Compilation of Flood Data through 1974, McGlone Price, October 1979.
6. ———, U.S. Geological Survey, Water Supply Paper 2207, Flood Characteristics of Urban Watersheds in the United States, Techniques for Estimating Magnitude and Frequency of Urban Floods, V. B. Sauer, W. O. Thomas, Jr., V. A. Stricker, and K. V. Wilson, 1983.
7. Post, Buckley, Schuh & Jernigan, Inc., Hydrology Study for Aragon, Rockmart and Unincorporated Polk County, Atlanta, Georgia, May 1985.
8. Woolpert Consultants, Aerial Photography, Polk County, Georgia, Scale 1:14400, Mobile, Alabama, December 1984.
9. U.S. Army Corps of Engineers, Hydrologic Engineering Center, HEC-2 Water-Surface Profiles, Generalized Computer Program, Davis, California, April 1984.
10. Woolpert Consultants, Topographic Mapping, Polk County, Georgia, Scale 1:4800, Contour Interval 4 Feet: Mobile, Alabama, 1985.

ELEVATION REFERENCE MARKS

REFERENCE MARK	FLOOD INSURANCE RATE MAP PANEL	ELEVATION (FEET NGVD)	DESCRIPTION OF LOCATION
1	0001	777.160	chiseled cross on top of concrete abutment at end of handrail on southwest corner of West Avenue bridge over Cedar Creek
2	0001	777.669	U.S. Coast and Geodetic Survey disk set in top of west end of north headwall of concrete box culvert at northeast corner of intersection of State Route 6 and Prior Station Road
3	0001	781.131	cross on top of steel stringer at southwest corner of CSX railroad trestle over Cedar Creek about 1,000 feet east of intersection of railroad and Second Street
4	0001	790.090	chiseled cross on top west end of north headwall of concrete box culvert for South Prong Skeeter Branch under East Avenue, about 100 feet west of intersection of East Avenue and Collard Valley Road
5	0001	790.661	railroad spike in power pole located on south side of Plantation Avenue at intersection of Plantation Avenue and John Hand Drive
6	0001	802.506	railroad spike set in power pole located on west side of North Martiele Street, about 1,150 feet north of intersection of North Martiele Street and East Avenue
7	0001	769.913	chiseled cross on top of wheel guard at northwest corner of concrete box culvert for Skeeter Branch under North Cave Spring Road about 105 feet north of intersection of North Cave Spring Road and Jule Peek Avenue
8	0001	774.473	chiseled cross on north end of west headwall at bottom of handrail on culvert for Skeeter Branch under College Street

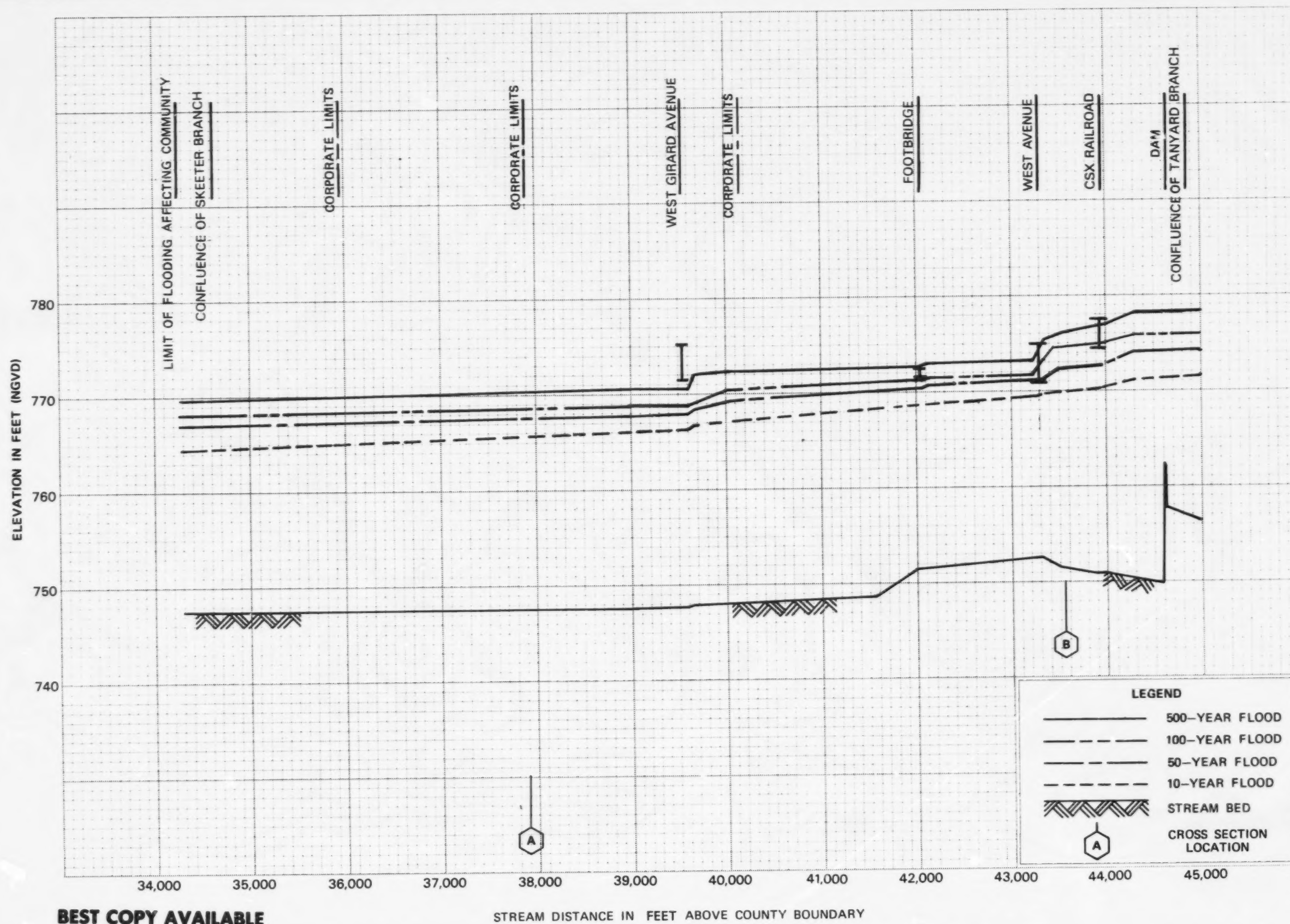
ELEVATION REFERENCE MARKS (continued)

REFERENCE MARK	FLOOD INSURANCE RATE MAP PANEL	ELEVATION (FEET NGVD)	DESCRIPTION OF LOCATION
9	0001	776.079	chiseled cross on top of north concrete headwall of culvert on West Girard Avenue, 167 feet east of intersection of West Girard Avenue and Marshall Street
10	0001	790.340	U.S. Coast and Geodetic Survey disk "R-197-148" set in top of south end of west headwall of concrete box culvert located on U.S. Route 27 about 75 feet north of north entrance drive into Haney's Tourist Court, 23 feet west of centerline of U.S. Route 27
11	0001	778.803	chiseled cross in northwest wingwall of North Main Street bridge over Skeeter Branch
12	0001	786.123	quarter-inch nail set in power pole located at northeast corner of intersection of Hillside Drive and Parkwood Lane
13	0001	786.168	railroad spike set in wood abutment at southwest corner of Norfolk Southern Railway trestle over Skeeter Branch
14	0001	786.920	railroad spike set in wood stringer at center of east side of Norfolk Southern Railway trestle located about 330 feet north of intersection of railway and Jule Peek Avenue
15	0002	774.973	chiseled square in concrete wheel guard at northeast corner of Canal Street bridge over Cedar Creek
16	0002	780.185	railroad spike set in power pole located on west side of Georgia Avenue about 200 feet north of intersection of Georgia Avenue and Old Battletown Road
17	0002	762.445	chiseled cross on west end of concrete dam on Cedar Creek located about 750 feet south of CSX railroad bridge over Cedar Creek

ELEVATION REFERENCE MARKS (continued)

REFERENCE MARK	FLOOD INSURANCE RATE MAP PANEL	ELEVATION (FEET NGVD)	DESCRIPTION OF LOCATION
18	0002	773.814	chiseled square on top southwest wingwall of Russel Street bridge over Tanyard Branch
19	0002	781.373	chiseled cross on top of concrete handrail at southeast corner of South Main Street bridge over Tanyard Branch
20	0002	783.074	chiseled cross on top northwest corner of box culvert headwall for Tanyard Branch under Rock Street
21	0002	783.138	railroad spike set in wood stringer at northwest corner of Norfolk Southern Railway trestle over Tanyard Branch
22	0002	793.719	railroad spike set in power pole located on north side of Philpot Spring Road, about 500 feet west of intersection of Philpot Spring Road and Pinecrest Road
23	0002	803.091	chiseled cross on top of concrete abutment at southeast corner of CSX railroad trestle over Lake Street
24	0002	799.390	railroad spike set in telephone pole no. 362 located on south side of CSX railroad, about 1,050 feet east of railroad trestle over Lake Street

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FLOOD PROFILES

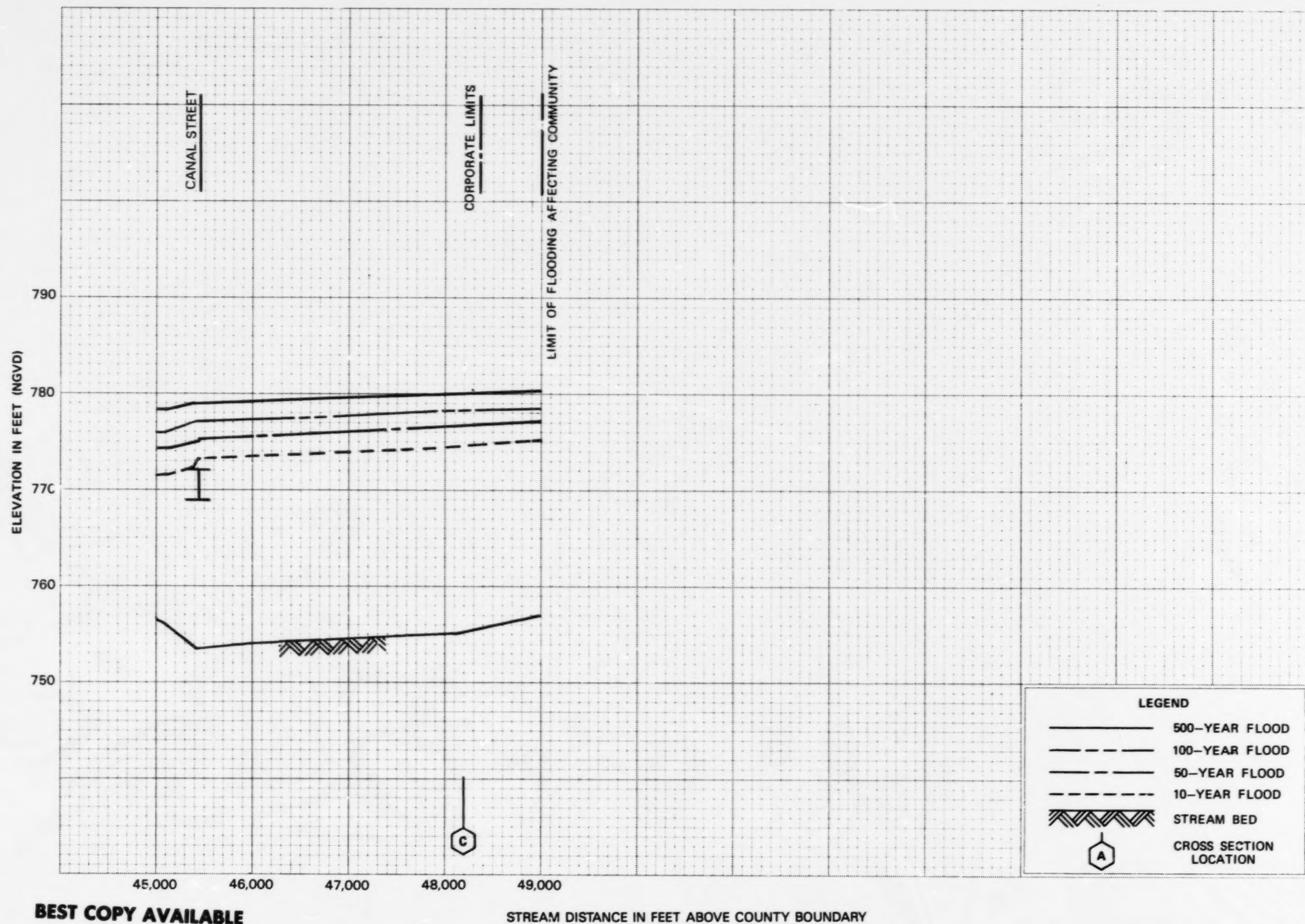
CEDAR CREEK

FEDERAL EMERGENCY MANAGEMENT AGENCY

CITY OF CEDARTOWN, GA

(POLK CO.)

01P



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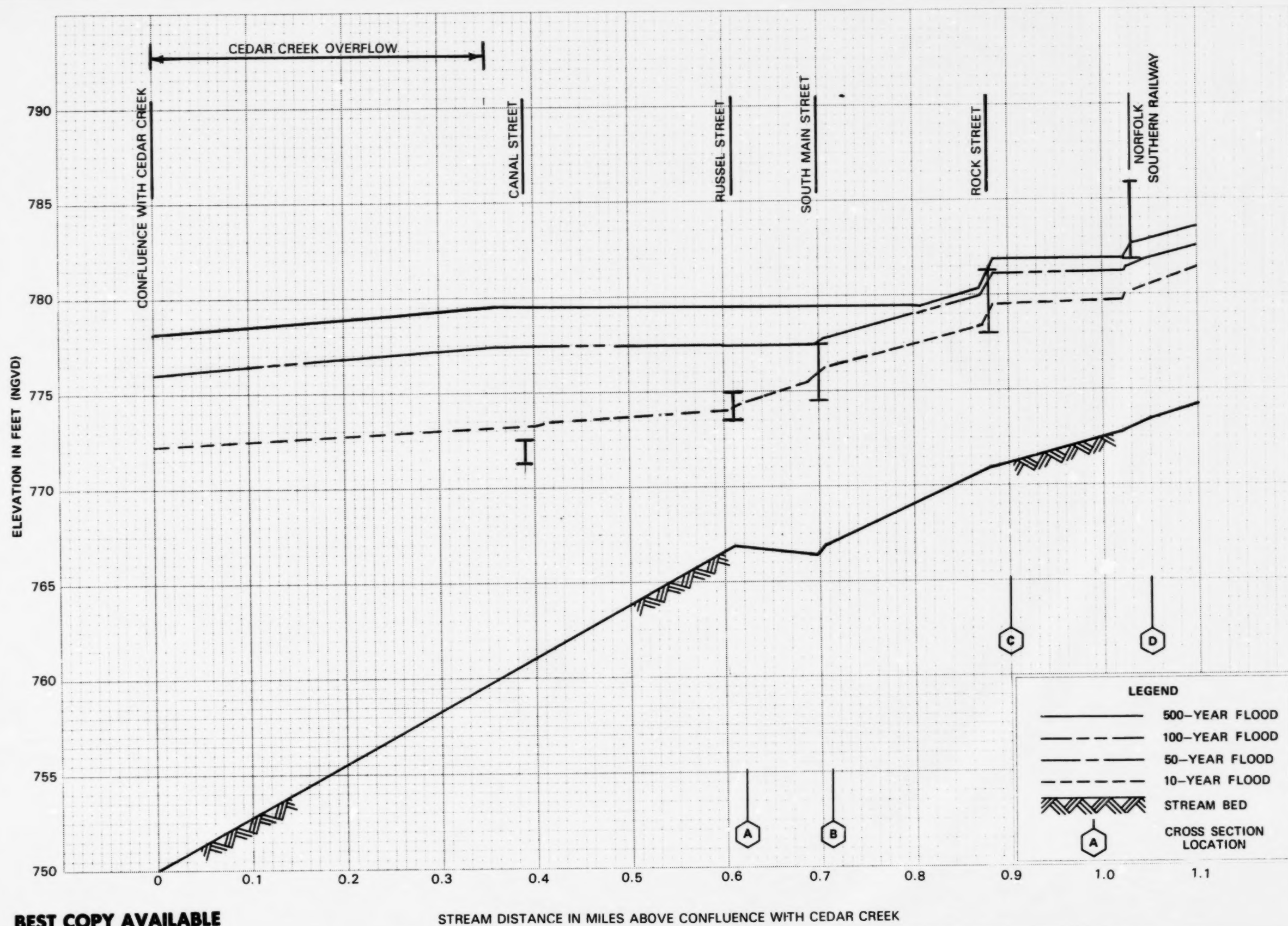
CEDAR CREEK

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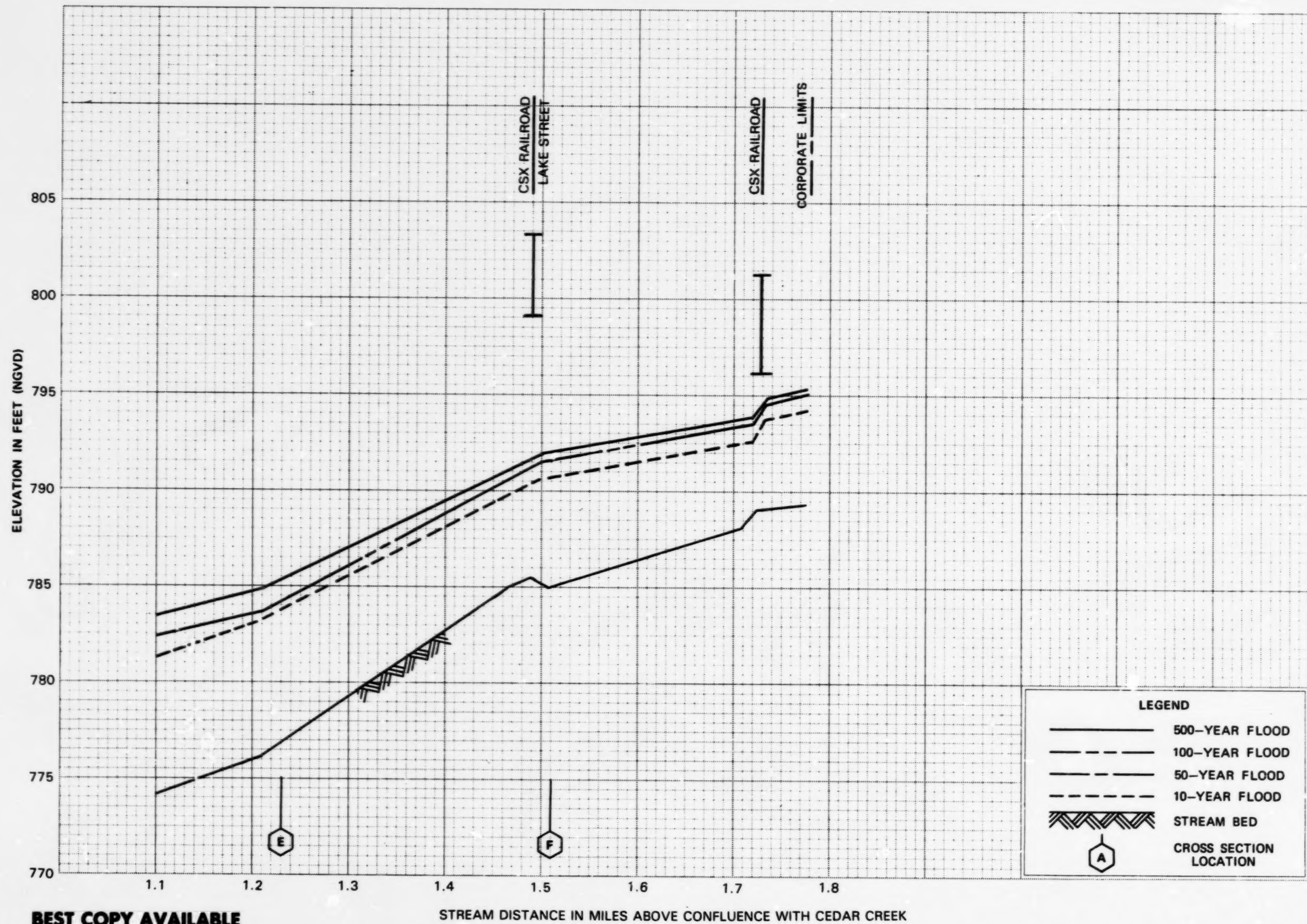
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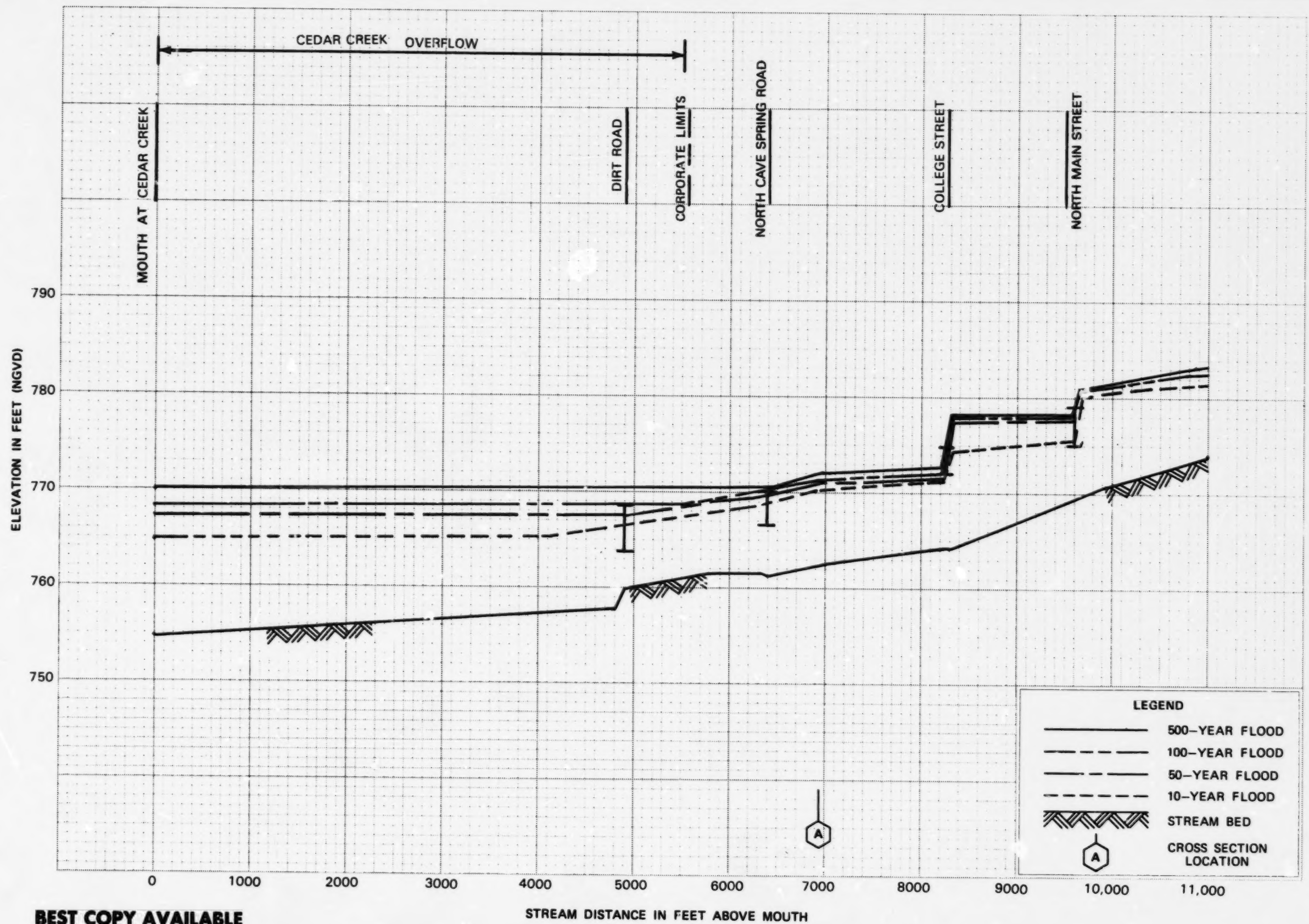
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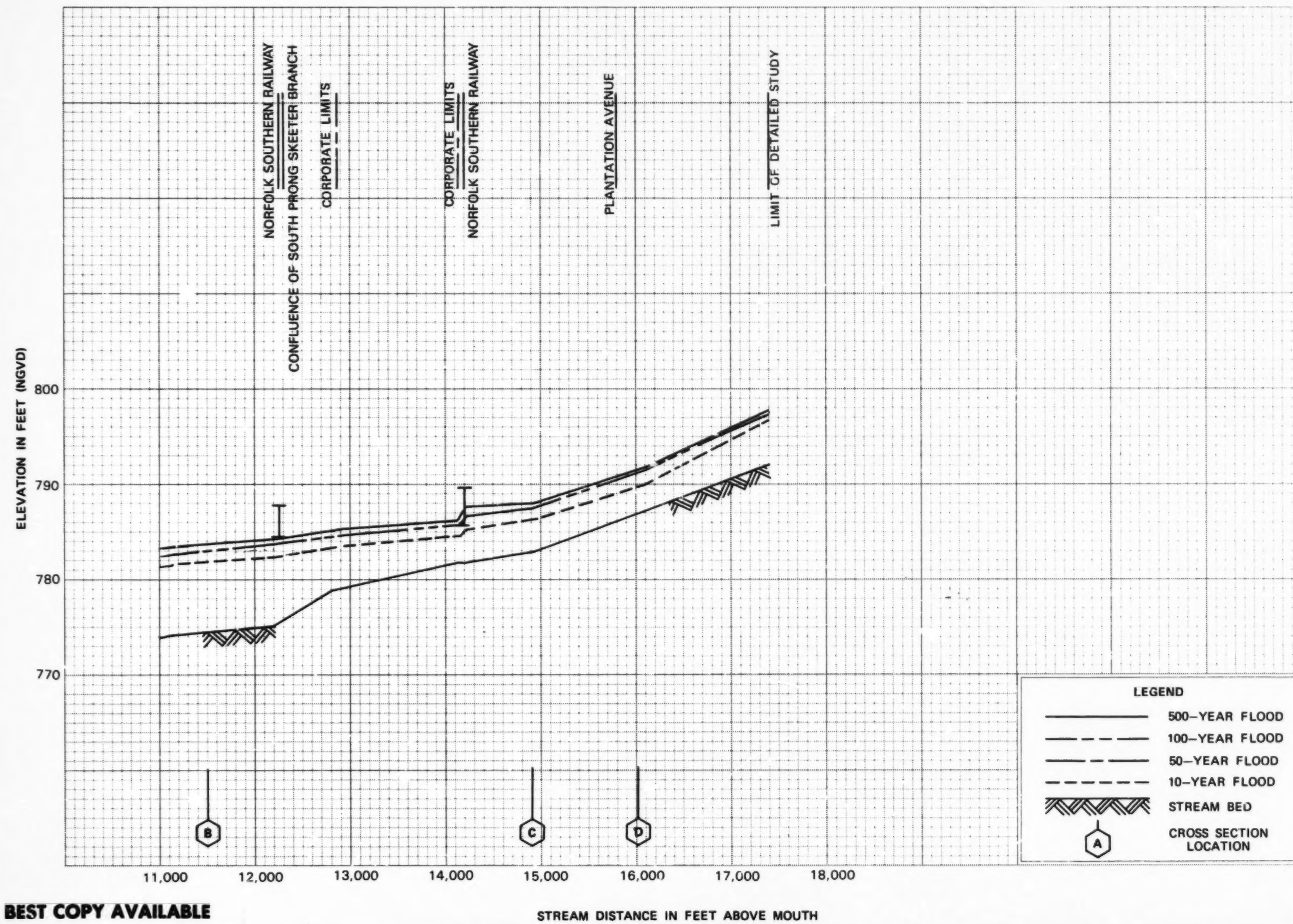
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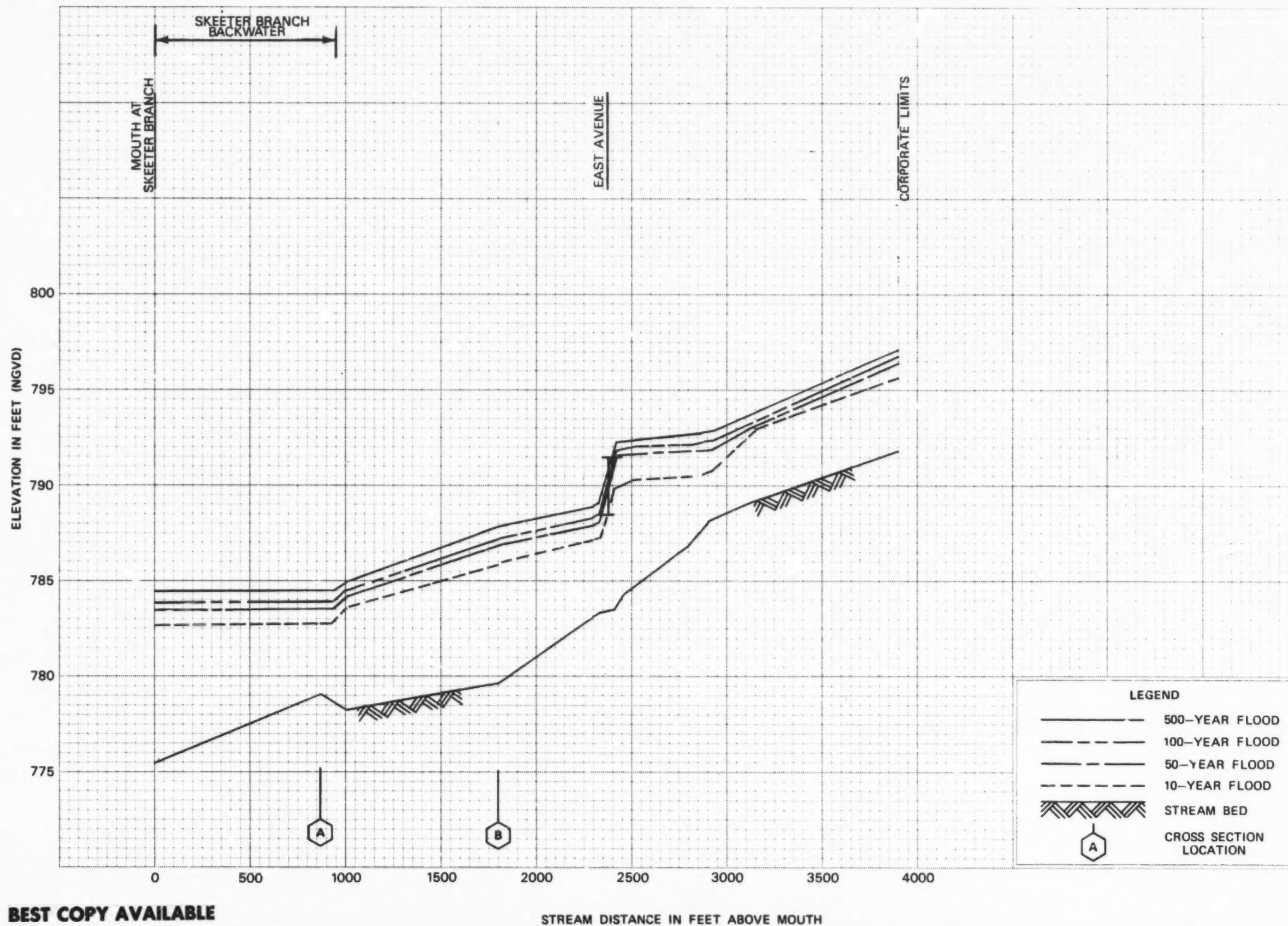
SKEETER BRANCH

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